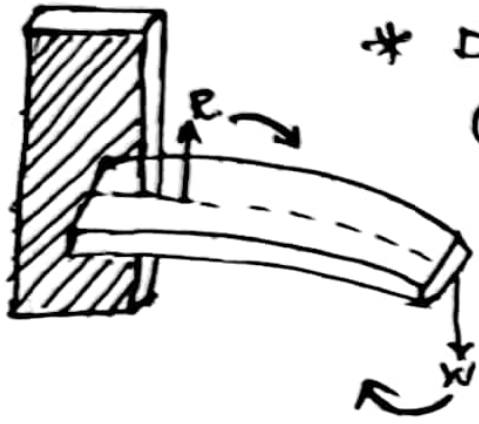


Cantilever: A cantilever is a light beam fixed horizontally at one end and loaded at the other end.



\* Due to load applied at the free end a couple is created b/w the two forces

(i) Force (load  $W$ ) applied at the free end towards  $\downarrow$  and direction.

(ii) Reaction ( $R$ ) acting in the  $\uparrow$ ward direction at the fixed end.

\* Internal bending couple tends to bend the beam in the clockwise direction.

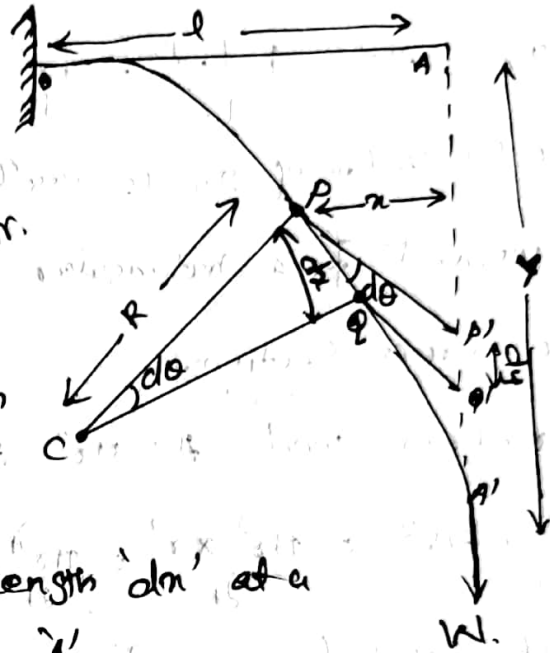
\* One end of the beam is fixed and the beam cannot rotate,  $\therefore$  External bending couple must be balanced by another equal & opposite couple created due to the elastic nature of the body (i.e.) called Int. bending moment

## Under Equilibrium Condition:

Ext. Bending Moment = Int. Bending Moment

## Depression of Cantilever - Loaded at its ends:-

- $l$  - length of the cantilever  
OA fixed at 'O'.
- $W$  - weight suspended at the loaded end 'A' of the cantilever.
- Due to the load applied the cantilever moves to a new position OA'.
- Consider small element PQ - length 'dm' at a distance 'n' from the free end A'.
- C - Center of curvature of the element PQ.
- R - Radius of curvature.



- Due to load applied at the free end of the cantilever an ext. couple is created b/w the load  $W$  at A' & the force of reaction at P.

$\therefore$  Ext. bending moment (or) mo. of the force about the element PQ  $\int = W \cdot n \rightarrow \textcircled{1}$

Wkt, the Int. Bending moment =  $\frac{EI\theta}{R} \rightarrow \textcircled{2}$

For Equilibrium: Ext. Bending moment = Int. Bending moment.

$$W \cdot n = \frac{EI\theta}{R}$$

$$\therefore R = \frac{EI\theta}{W \cdot n} \rightarrow \textcircled{3}$$

- Radius of ~~the~~ curvature of the neutral axis varies from point to point of the cantilever, the bending is said to be non-uniform.
- Now, two tangents are drawn at the points P & Q which meet the vertical line AA' at P' & Q'.
- Smallest depression produced from P' to Q' = dy.
- Angle b/w the two tangents = dθ.
- Then we can write the angle b/w CP and CQ is also dθ.  
ie  $\angle PCQ = d\theta$  & the arc length PQ =  $Rd\theta = dx$ .

$$\therefore d\theta = \frac{dx}{R} \rightarrow (4)$$

Sub equ (3) in eqn (4) we get.

$$d\theta = \frac{dx}{\left(\frac{EI}{W}\right)}$$

$$\therefore d\theta = \frac{W \cdot dx}{EI} \rightarrow (5)$$

$$P'Q' = r \cdot d\theta$$

$$dy = r \cdot d\theta \rightarrow (6)$$

Sub equ (5) in eqn (6) we get the small depression produced by the element PQ.

$$dy = r \cdot \frac{W \cdot dx}{EI} \rightarrow (7)$$

$\therefore$  Total depression at the free end of the cantilever can be derived by  $\int$  in the eqn (7) within the limits '0 - l'

$$\therefore y = \frac{W}{EI} \int_0^l r^2 dx$$

$$= \frac{W}{EI} \left[ \frac{r^3}{3} \right]_0^l$$

$$= \frac{W}{EI} \left[ \frac{l^3}{3} \right]$$

$$\left[ \int \frac{r^{n+1}}{n+1} = \int r^n dx \right]$$

∴ The total depression of the cantilever at the free end

$$y = \frac{wl^3}{3EI_g} \rightarrow \textcircled{a}$$

∴ Young's modulus of the material of the cantilever

$$E = \frac{wl^3}{3yI_g} \text{ a.}$$

### Special Cases:

(i) Rectangular Cross Sectional Beam.

- b - Breadth
- d - Thickness

$$\therefore I_g = \frac{bd^3}{12}$$

$$y = \frac{wl^3}{3E(bd^3/12)}$$

$$y = \frac{4wl^3}{Ebd^3}$$

(ii) Circular Cross sectional Beam:

- r - Radius of the circular section of the beam

$$\therefore I_g = \frac{\pi r^4}{4}$$

$$\therefore y = \frac{wl^3}{3E(\pi r^4/4)}$$

$$y = \frac{4wl^3}{3\pi r^4 E}$$

