

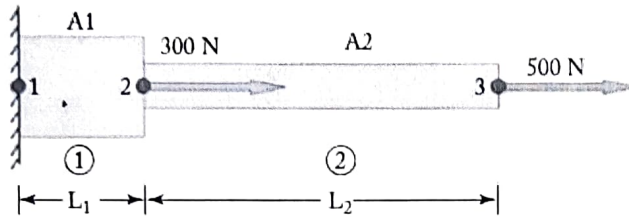


DEPARTMENT OF MECHANICAL ENGINEERING

16ME401 Finite Element Analysis

UNIT II One Dimensional Problems

A compound axial member is subjected to the loads shown in Fig. Given,

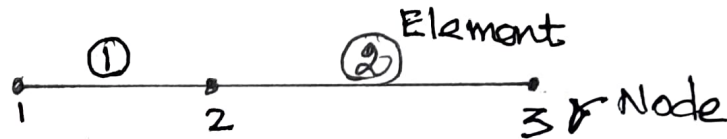


Youngs modulus  $E_1 = 50MN/m^2, E_2 = 100MN/m^2,$   
 $E_1 = 50 \times 10^6 N/m^2$   
 $E_2 = 100 \times 10^6 N/m^2$   
 Length  $L_1 = 0.5m, L_2 = 1m, Area A_1 = 20cm^2, A_2 = 10cm^2$   
 $A_1 = 0.002 m^2$   
 $A_2 = 0.001 m^2$

Figure compound axial member subjected to loads.

Calculate the following: (i) Nodal displacement (ii) Element strain (iii) Element stresses (iv) Support reaction, (v) Compare with exact solution, Using two bar elements model. **FEA Model**

Solution:



Number of elements = 2; and number of nodes = 3.

The stiffness matrix of each element is computed from

$$[K]^{(e)} = \frac{AE}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

A - Area of the Element  
 E - Youngs modulus of the Element  
 L - Length of the Element

For element (1), the stiffness matrix is

$$[K]^{(1)} = \frac{A_1 E_1}{L_1} \Rightarrow [K]^{(1)} = \frac{0.002 \times (50 \times 10^6)}{0.5} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[K]^{(1)} = 2 \times 10^5 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[K]^{(1)} = 10^5 \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix}$$

①

2 Node Displacement direction

For element (2), the stiffness matrix is

$$[K]^{(2)} = \frac{A_2 E_2}{L_2} \Rightarrow [K]^{(2)} = \frac{0.001 \times (100 \times 10^6)}{1} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$[K]^{(2)} = 10^5 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \quad [K]^{(2)} = 1 \times 10^5 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

The final global matrices of stiffness, displacement, and load matrix, are obtained by combining the element matrices as

$$[K]^G = 10^5 \begin{bmatrix} 2 & -2 & 0 \\ -2 & 2+1 & -1 \\ 0 & -1 & 1 \end{bmatrix} \begin{matrix} 1 \\ 2 \\ 3 \end{matrix}; \{u\} = \begin{Bmatrix} u_1 \\ u_2 \\ u_3 \end{Bmatrix}; \{F\} = \begin{Bmatrix} 0 \\ 300 \\ 500 \end{Bmatrix}$$

$u_1 = 0$

Applying the fixed boundary conditions at node 1 and applying the external forces at nodes 2 and 3, we have

$$10^5 \begin{bmatrix} 2 & -2 & 0 \\ -2 & 2+1 & -1 \\ 0 & -1 & 1 \end{bmatrix} \begin{matrix} 1 \\ 2 \\ 3 \end{matrix} \begin{Bmatrix} 0 \\ u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 300 \\ 500 \end{Bmatrix}$$

Using the matrix partitioning to solve for  $u_2$  and  $u_3$ , we have

$$10^5 \begin{bmatrix} 3 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} 300 \\ 500 \end{Bmatrix} \Rightarrow 10^5(3u_2 - u_3) = 300 \quad \text{--- (1)}$$

$$10^5(-u_2 + u_3) = 500$$

$$\begin{Bmatrix} u_2 \\ u_3 \end{Bmatrix} = \begin{Bmatrix} 0.004 \\ 0.009 \end{Bmatrix} \text{ m.}$$

Solve eqn (2) by gaussian elimination approach.

(i) Nodal displacement

$$u_2 = 0.004 \text{ m} \quad u_3 = 0.009 \text{ m}$$

(ii) Nodal Element Strain

$$\text{Element (1)} \quad \epsilon^{(1)} = \frac{u_2 - u_1}{L_1} = \frac{0.004 - 0}{0.5} = 0.008$$

$$\text{Element (2)} \quad \epsilon^{(2)} = \frac{u_3 - u_2}{L_2} = \frac{0.009 - 0.004}{1} = 0.005$$

(iii) Element Stresses [Found by Hooke's law]

$$\sigma^{(1)} = E_1 \times \epsilon^{(1)} = 50 \times 10^6 \times 0.008 = 4 \times 10^5 \text{ N/m}^2$$

$$\sigma^{(2)} = E_2 \times \epsilon^{(2)} = 100 \times 10^6 \times 0.005 = 5 \times 10^5 \text{ N/m}^2$$

⇒ Theoretical stress

$$\sigma^{(1)} = P_1/A_1 = \frac{800}{0.002} = 4 \times 10^5 \text{ N/m}^2$$

$$\sigma^{(2)} = P_2/A_2 = 500/0.001 = 5 \times 10^5 \text{ N/m}^2$$

System of Equations.

$$10^5 \begin{bmatrix} 2 & -2 & 0 \\ -2 & 3 & -1 \\ 0 & -1 & 1 \end{bmatrix} \begin{bmatrix} 0 \\ 0.004 \\ 0.009 \end{bmatrix} = \begin{bmatrix} R_1 \\ 300 \\ 500 \end{bmatrix}$$

Using the first row of the global matrix to find  $R_1$ , we have

(iv) Support reaction  $[R_1]$

$$R_1 = -2 \times 10^5 \times 0.004 = -800 \text{ N.}$$

Check for Equilibrium: Action Forces = - Reaction Forces

$$F_2 + F_3 = 300 + 500 = 800 \text{ N} \quad R_1 = -800 \text{ N.}$$

Result;

(i) Nodal displacement

$$u_1 = 0 \quad u_2 = 0.004 \text{ m} \\ u_3 = 0.009 \text{ m}$$

(iii) Element stresses

$$\sigma^{(1)} = 4 \times 10^5 \text{ N/m}^2 \\ \sigma^{(2)} = 5 \times 10^5 \text{ N/m}^2$$

(v) Compare with exact solution

- Verified. Same result obtained.

(ii) Element strain

$$\epsilon^{(1)} = 0.008 \\ \epsilon^{(2)} = 0.005$$

(iv) Support reaction

$$R_1 = -800 \text{ N}$$