

Solution

Given

$$\sigma_z(\max) = 140 \text{ MPa} \Rightarrow 140 \text{ N/mm}^2.$$

$$R_i = 25 \text{ mm.}$$

$$R_o = 25 + 25 = 50 \text{ mm.}$$

$$b_i = 19 \text{ mm.}$$

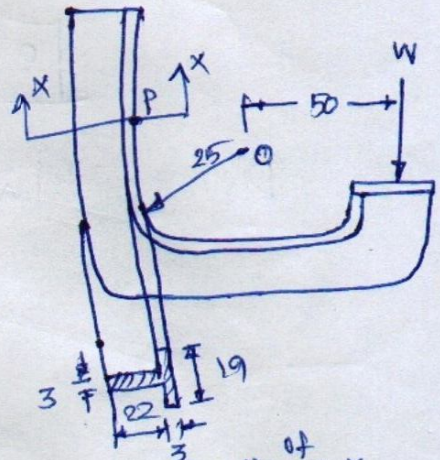
$$t_i = 3 \text{ mm.}$$

$$t = 3 \text{ mm.}$$

$$h = 25 \text{ mm.}$$

Wkt, the area of section at x-x

$$A = 3 \times 22 + 3 \times 19 = \underline{\underline{123 \text{ mm}^2}}$$



All dimensions in mm

Wkt, Radius of curvature of the neutral axis,

$$R_n = \frac{(b_i - t) t_i + (t \times h)}{}$$

$$\frac{(b_i - t) \log_e \left[ \frac{R_i + t_i}{R_i} \right] + t \log_e \left[ \frac{R_o}{R_i} \right]}{}$$

From PSG  
DPB Pg-no  
6.3.

$$R_n = \frac{3(19-3)3 + (3 \times 25)}{}$$

$$(19-3) \log_e \left[ \frac{25+3}{25} \right] + 3 \log_e \left[ \frac{50}{25} \right]$$

$$R_n = \frac{123}{16 \times 0.113 + 3 \times 0.693} = \frac{123}{3.887} = \underline{\underline{31.64 \text{ mm}}}$$

Radius of curvature of the ~~net~~ central axis

$$R = \frac{R_i + \frac{1}{2} h^2 - t + \frac{1}{2} t_i^2 (b_i - t)}{h \cdot t + t_i (b_i - t)}$$

$$= \frac{25 + \frac{1}{2} \times 25^2 \times 3 + \frac{1}{2} \times 3^2 (19-3)}{25 \times 3 + 3(19-3)} = 25 + \frac{937.5 + 72}{75 + 48}$$

$$R = 25 + 8.2 \Rightarrow \underline{\underline{R = 33.2 \text{ mm.}}}$$

Distance b/w central axis to the neutral axis,

$$e = R - R_n = 33.2 - 31.64$$

$$e = 1.56 \text{ mm.}$$

Distance b/w the load  $w$  and the centroidal axis,

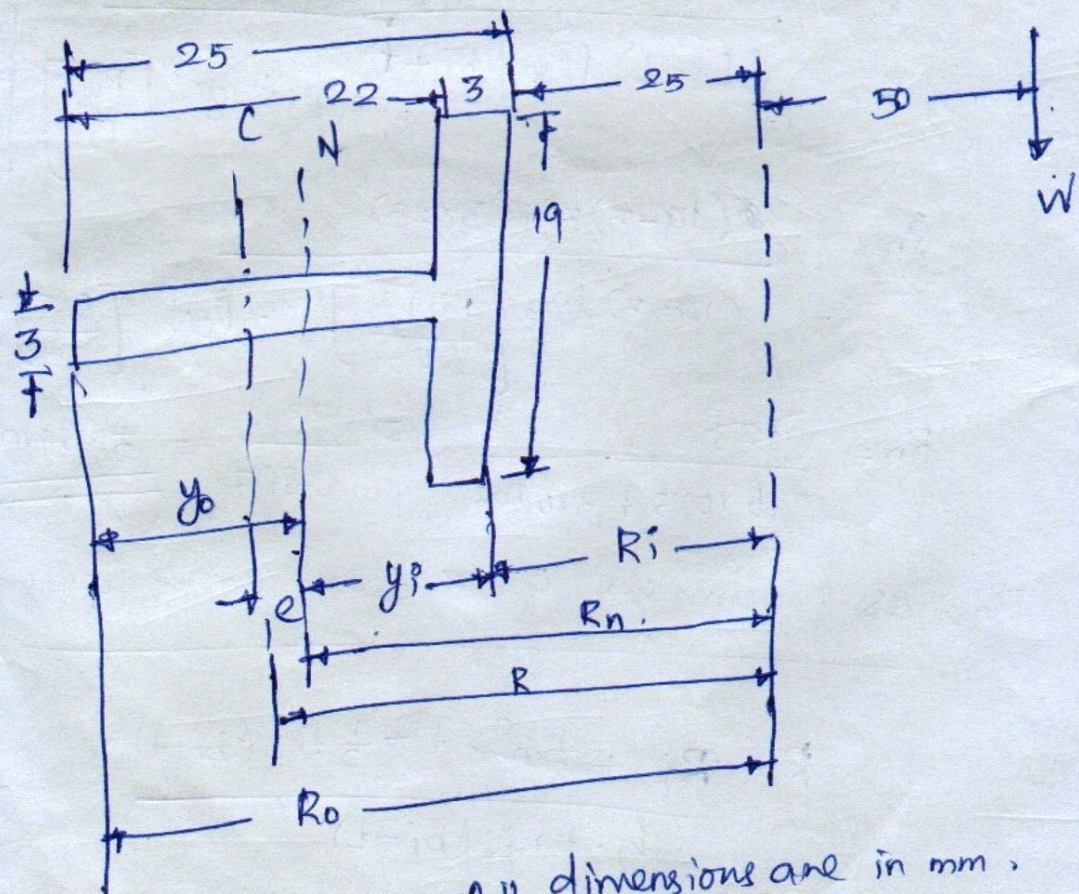
$$x = 50 + R = 50 + 33.2 = 83.2 \text{ mm.}$$

$$x = 83.2 \text{ mm.}$$

$\therefore$  Bending moment about the centroidal axis,

$$M = W \cdot x \Rightarrow W \times 83.2 \Rightarrow 83.2 W \text{ N-mm}$$

$$M = 83.2 W \text{ N-mm}$$



All dimensions are in mm.

The section at x-x is subjected to a direct tensile load of  $W$  and a bending moment of  $83.2W$ .

The max. tensile stress will occur at point P. (i.e. ~~at~~ the <sup>inside</sup> inner fibre of the section).

Distance from the neutral axis to the point P,

$$y_i = R_n - R_i = 31.64 - 25 = \underline{\underline{6.64 \text{ mm}}}$$

Direct tensile stress at section x-x

$$\sigma_t = \frac{W}{A} = \frac{W}{123} = \underline{\underline{0.008 W \text{ N/mm}^2}}$$

Max. bending stress at point P,

$$\sigma_{bi} = \frac{M \cdot y_i}{A \cdot R_i} = \frac{83.2W \times 6.64}{123 \times 1.56 \times 25}$$

$$\boxed{\sigma_{bi} = 0.115 W \text{ N/mm}^2}$$

We know that max. tensile stress  $\sigma_t(\text{max})$

Resultant stress =  $\sigma_t + \sigma_{bi}$ .

$$140 = \sigma_t + \sigma_{bi} = 0.008W + 0.115W = \underline{\underline{0.123W}}$$

$$W = 140 / 0.123 = \underline{\underline{1138 \text{ N}}}$$

$$\boxed{W = 1138 \text{ N}}$$

Note: -

wk.t, the distance from the neutral axis to the outer fibre,

$$y_o = R_o - R_n = 50 - 31.64 = \underline{\underline{18.36 \text{ mm}}}$$

∴ Max. bending stress at the outer fibre,

$$\sigma_{bo} = \frac{M \cdot y_o}{A \cdot e \cdot R_o} = \frac{83.2W \times 18.36}{123 \times 1.56 \times 50} = \underline{\underline{0.16W}}$$

Max. stress at outer fibre,

$$= \sigma_t - \sigma_{b0} = 0.008W - 0.16W$$

$$= -0.152W \text{ N/mm}^2$$

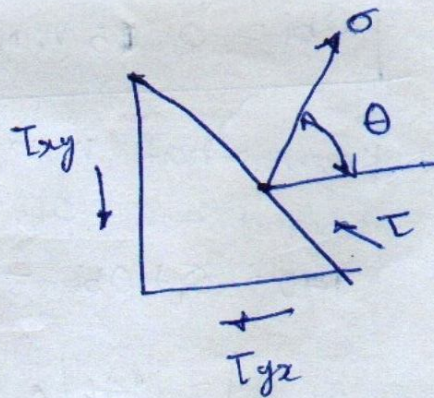
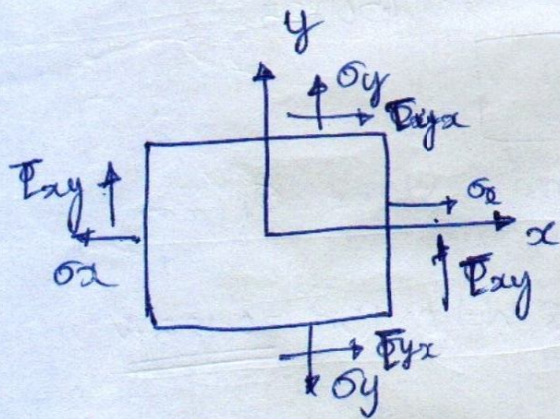
$$= \underline{\underline{0.152W \text{ N/mm}^2}} \text{ (compressive)}$$

From ~~the~~ above we see that stress at the outer fibre is larger in this case than at the inner fiber, but this stress at outer fibre is compressive.

### Principal stresses for variable load combinations

Normal stress,  $\sigma$

Tangential stress,  $\tau$ .



$$\sigma_1 = \frac{\sigma_x + \sigma_y}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2}$$

$$\sigma_2 = \frac{\sigma_x + \sigma_y}{2} - \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2}$$

Max. shear stress

$$\tau_{\max} = \frac{1}{2} \sqrt{(\sigma_x - \sigma_y)^2 + 4\tau_{xy}^2}$$

$$\tan 2\theta = \frac{2\tau_{xy}}{\sigma_x - \sigma_y}$$

Max. Principle Stress

$$\sigma_1 = \frac{\sigma_x + \sigma_y}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} \Rightarrow \frac{\sigma_x + \sigma_y}{2} + \frac{1}{2} \sqrt{(\sigma_x - \sigma_y)^2 + 4\tau_{xy}^2}$$

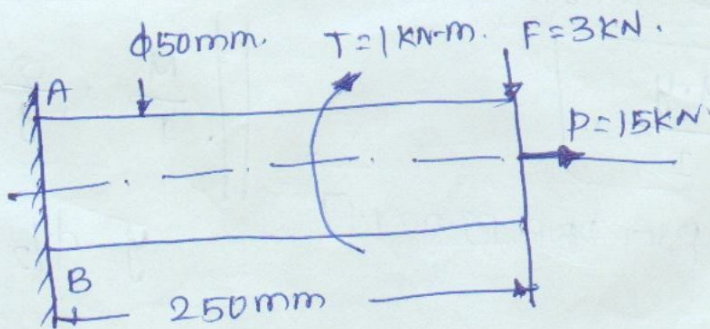
Min principle stress

$$\sigma_2 = \frac{\sigma_x + \sigma_y}{2} - \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} \Rightarrow \frac{\sigma_x + \sigma_y}{2} - \frac{1}{2} \sqrt{(\sigma_x - \sigma_y)^2 + 4\tau_{xy}^2}$$

$$\text{Max. Shear Stress} = \tau_{\max} = \frac{\sigma_1 - \sigma_2}{2}$$

Problems:

Calculate the normal stresses at (A) & (B), and also calculate max. shear stresses at (A) and (B).



Solution:-

Given

$$d = 50 \text{ mm}, \quad T = 1 \text{ kN-m} \Rightarrow 1 \times 10^3 \text{ N-m}, \quad F = 3 \text{ kN} = 3000 \text{ N}$$

$$P = 15 \text{ kN} = 15 \times 10^3 \text{ N}, \quad l = 250 \text{ mm}$$

i) consider axial load.

Axial load includes direct stress.

$$\sigma_t = \text{Direct stress} = \frac{P}{A} = \frac{15 \times 10^3}{1963.49}$$

$$\sigma_t = 7.639 \text{ N/mm}^2$$

$$P = \text{Axial load} = \text{Tensile} = 15 \text{ kN} \\ = +15 \times 10^3 \text{ N}$$

$$A = \frac{\pi}{4} d^2 = \frac{\pi}{4} \times 50^2 = 1963.49 \text{ mm}^2$$

ii) consider ~~trans~~ transverse load bending load.

$$\text{Bending load} = F = 3 \text{ kN} = 3 \times 10^3 \text{ N}$$

$$\text{Bending moment} = \text{force} \times \text{length} = 3 \times 10^3 \times 250 \\ = 750 \times 10^3 \text{ N-mm}$$

Bending load includes bending stress.

$$\sigma_b = \frac{M \cdot y}{I}$$

[PSEB DDB pg. 007.1]

$$\frac{M}{I} = \frac{\sigma}{y} \quad \sigma_b = \frac{M \cdot y}{I}$$

$$y = d/2 = 50/2 = 25 \text{ mm}$$

$$\sigma_b = \frac{750 \times 10^3 \times 25}{306.79 \times 10^3}$$

$$I = \frac{\pi d^4}{64}$$

$$I = \frac{\pi}{64} \times (50)^4$$

$$I = 306.79 \times 10^3 \text{ mm}^4$$

$$\sigma_b = +61.115 \text{ N/mm}^2 \text{ (tensile)}$$

$$\sigma_b = -61.115 \text{ N/mm}^2 \text{ (compressive)}$$

$$\text{Total stress} = \sigma_x = \sigma_t + \sigma_b$$

at A

$$\sigma_x = \sigma_t + (\sigma_b) t = +7.639 + 61.115 = \underline{\underline{68.75 \text{ N/mm}^2}}$$

at B

$$\sigma_x = \sigma_t + (\sigma_b) c = +7.639 + (-61.115) = \underline{\underline{-53.476 \text{ N/mm}^2}}$$

(iii). Consider torsion

$$\text{Twisting moment (T)} = 1 \text{ kN-m} = \underline{\underline{1000 \times 10^3 \text{ N-mm}}}$$

$$\frac{T}{J} = \frac{\tau}{r} \quad [\text{p. 6 DDB Pg. no 7.1}]$$

$$\tau_{xy} = \tau = \frac{T \cdot r}{J} \\ = \frac{1 \times 10^6 \times 25}{613.59 \times 10^3}$$

$$r = \frac{50}{2} = 25 \text{ mm.}$$

$$J = \frac{\pi}{32} d^4$$

$$J = \frac{\pi}{32} \times 50^4$$

$$J = \underline{\underline{613.59 \times 10^3 \text{ mm}^4}}$$

$$\boxed{\tau_{xy} = \tau = 40.74 \text{ N/mm}^2}$$

At (A)

$$\sigma_x = \underline{\underline{+68.75 \text{ N/mm}^2}}$$

$$\sigma_y = 0.$$

$$\tau_{xy} = \underline{\underline{+40.74 \text{ N/mm}^2}}$$

At (B)

$$\sigma_x = \underline{\underline{-53.476 \text{ N/mm}^2}}$$

$$\sigma_y = 0$$

$$\tau_{xy} = \underline{\underline{+40.74 \text{ N/mm}^2}}$$

at (A)

$$\sigma_1 = \text{Max. normal stress} = \frac{\sigma_x + \sigma_y}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$= \frac{68.75}{2} + \sqrt{\left(\frac{68.75}{2}\right)^2 + (40.74)^2}$$

$$= 34.375 + 53.30$$

$$\boxed{\sigma_1 = +87.675 \text{ N/mm}^2}$$

$$\sigma_2 = \text{Min. normal stress} = \frac{\sigma_x + \sigma_y}{2} - \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$= 34.375 - 53.30$$

$$\boxed{\sigma_2 = -18.925 \text{ N/mm}^2}$$

$$\text{Max. shear stress} = (\tau_{\text{max}})_{\text{at A}} = \frac{\sigma_1 - \sigma_2}{2}$$

$$= \frac{87.675 - (-18.925)}{2}$$

$$\boxed{(\tau_{\text{max}})_{\text{at A}} = 53.3 \text{ N/mm}^2}$$

At B:-

$$\sigma_f = \text{Max. normal stress} = \frac{\sigma_x + \sigma_y}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$= \frac{-53.476}{2} + \sqrt{\left(\frac{-53.476}{2}\right)^2 + (40.74)^2}$$

$$= -26.738 + 48.730$$

$$\boxed{\sigma_f = +21.992 \text{ N/mm}^2 \text{ (tensile)}}$$



